

**BIGGS CARDOSA
ASSOCIATES INC**
STRUCTURAL ENGINEERS

5250 N Palm Ave, Suite 211
Fresno, CA 93704-2218
Telephone 559-449-8686
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STRUCTURAL CALCULATIONS

for

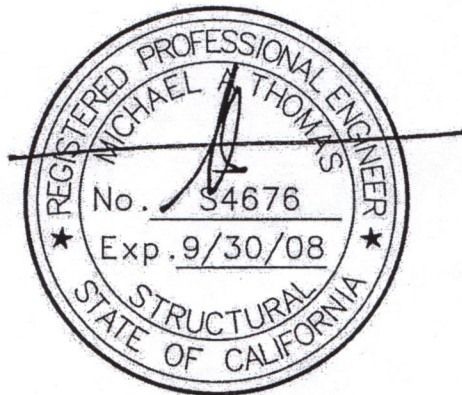
SELMA BRANCH LIBRARY

ANCHORING OF BOOK SHELVING

SELMA, CALIFORNIA

Prepared For:

County of Fresno
Department of Public Works
Capital Projects Division
2220 Tulare Street, 6th Floor
Fresno, CA 93721



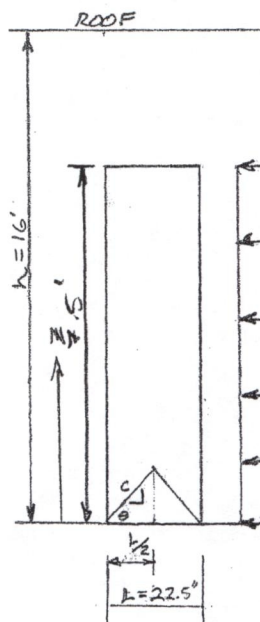
January 2008

BCA Job No. 2002025F

BCA

BOOK SHELVES

SEISMIC DESIGN FORCE



$$F_p = \frac{0.4 a_p S_{DS} W_p}{\frac{R_o}{I_p}} \left(1 + 2 \frac{z}{h} \right)$$

$$= \frac{0.4 \times 1 \times .408 \times W_p}{2.5} \left(1 + 2 \frac{7.5}{16} \right) = .12648 W_p$$

$$= .12648 \times \frac{17}{12} \times 3' \times 33 \frac{\#}{ft} = 17.8 \frac{\#}{ft}$$

$$F_p = \frac{0.4 \times 1 \times .408 \times W_p}{2.5} \left(1 + 2 \frac{0}{16} \right) = .06528 W_p$$

$$V = .06528 \times \frac{17}{12} \times 3 \times 33 \frac{\#}{ft} = 9.2 \frac{\#}{ft}$$

$$F_{p,max} = 1.6 S_{DS} I_p W_p = .6528 W_p = 9.2 \frac{\#}{ft}$$

$$F_{p,min} = 0.3 S_{DS} I_p W_p = .1224 W_p = 17.2 \frac{\#}{ft} \therefore$$

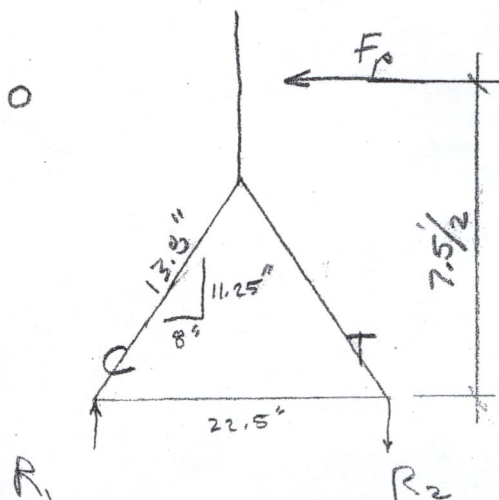
USE 17.8 PLF / PER 3' OF SHELVING

$$F_p @ \text{VENTRAL MEMBER} = 17.8 \text{ PLF} \times 7.5' \text{ HT} = 133.5 \frac{\#}{ft}$$

$$\sum M = - 3.75' F_p + 1.875' R_1 = 0$$

$$R_1 = \frac{3.75}{1.875} F_p = 267$$

$$C = \frac{13.8''}{11.25''} (267 \frac{\#}{ft}) = 328 \frac{\#}{ft}$$



FBD

Project Name = 2002025F Selma Library
 Conterminous 48 States
 2005 ASCE 7 Standard
 Latitude = 36.5687
 Longitude = -119.60934
 Spectral Response Accelerations Ss and S1
 Ss and S1 = Mapped Spectral Acceleration Values
 Site Class B - $F_a = 1.0$, $F_v = 1.0$
 Data are based on a 0.01 deg grid spacing
 Period S_a
 (sec) (g)
 0.2 0.512 (Ss, Site Class B)
 1.0 0.229 (S1, Site Class B)

OUTPUT FROM USGS JAVA
APPLET "SEISMIC HAZARD CURVES
AND UNIFORM HAZARD RESPONSE
SPECTRA"

Conterminous 48 States
 2005 ASCE 7 Standard
 Latitude = 36.5687
 Longitude = -119.60934
 Spectral Response Accelerations SMs and SM1
 SMs = $F_a S_s$ and SM1 = $F_v S_1$
 Site Class C - $F_a = 1.195$, $F_v = 1.571$

Period S_a
 (sec) (g)
 0.2 0.612 (SMs, Site Class C)
 1.0 0.359 (SM1, Site Class C)

Conterminous 48 States
 2005 ASCE 7 Standard
 Latitude = 36.5687
 Longitude = -119.60934
 SDs = $2/3 \times S_Ms$ and SD1 = $2/3 \times S_{M1}$
 Site Class C - $F_a = 1.195$, $F_v = 1.571$

Period S_a
 (sec) (g)
 0.2 0.408 (SDs, Site Class C)

STIFFENER PLATEGusset Capacity AnalysisDefinition of Units/Variables

$$\text{psi} := \frac{\text{lb}}{\text{in}^2} \quad \text{kip} := 1000 \cdot \text{lb} \quad \text{ksi} := \frac{\text{kip}}{\text{in}^2}$$

OK := "OK" NG := "No Good"

Plate Section Properties

$$\text{area} := 0.1868 \cdot \text{in}^2 \quad F_y := 33 \cdot \text{ksi} \quad E := 29000 \cdot \text{ksi}$$

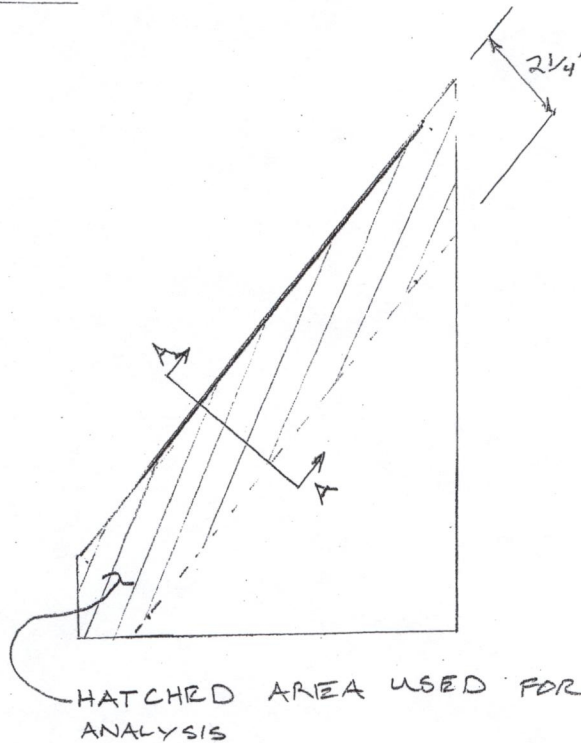
$$C_c := \sqrt{\frac{2 \cdot \pi^2 \cdot 29000 \cdot \text{ksi}}{F_y}} \quad C_c = 131.7$$

$$r := .0408 \cdot \text{in} \quad kl := 14 \cdot \text{in} \quad \frac{kl}{r} = 343.1$$

Compression Capacity

$$F_a := \left[\frac{12 \cdot \pi^2 \cdot E}{23 \cdot \left(\frac{kl}{r} \right)^2} \right] \quad F_a = 1.3 \text{ ksi}$$

$$P_{\text{allow}} := F_a \cdot \text{area} \quad P_{\text{allow}} = 236.9 \text{ lb}$$

CAPACITY VS DEMAND

$$236.9 \# \times 1.33$$

$$315 \#$$

$$> 230 \# \text{ OK}$$



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Title :
Dsgnr:
Description :

Job #
Date: 1:14PM, 19 NOV 07

4 OF 9

Scope :

Rev: 560100
User: KW-0602446, Ver 5.6.1, 25-Oct-2002
(c)1983-2002 ENERCALC Engineering Software

Built-Up Section Properties

Page 1

j:\active projects\2002\2002025\calcs and sk

Description 14 ga Plate (2 1/4" assumed depth)

General Information

Type...					X cg	Y cg
#1 Rectangular	Height	2.2500 in	Width	0.0747 in	0.0000 in	0.0000 in
#2 Rectangular	Height	0.0747 in	Width	0.2500 in	0.0900 in	1.1250 in

Summary

Total Area	0.1868 in ²	lxx	0.092 in ⁴	r xx	0.7026 in
X cg Dist.	0.0090 in	lyy	0.000 in ⁴	r yy	0.0408 in
Y cg Dist.	0.1125 in	Edge Distances from CG...			
		+X	0.2060 in	S left	0.0067 in ³
		-X	-0.0464 in	S right	0.0015 in ³
		+Y	1.0499 in	S top	0.0878 in ³
		-Y	-1.2375 in	S bottom	0.0745 in ³

Sketch & Diagram

Center of Gravity

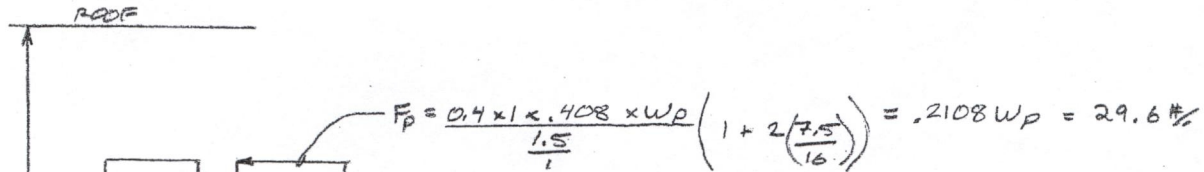
Datum

Center of Gravity Datum

SECTION AA

BOOK SHELVES

SEISMIC DESIGN FORCE



$R_p = 1.5$
 $A_p = 1.0$ (PER ASCE 7-05)
 CH 13.4

$F_p = \frac{0.4 \times 1 \times .408 \times W_p}{\frac{1.5}{1}} \left(1 + 2 \left(\frac{0}{16} \right) \right) = .1088 W_p = 15.3 \#/'$

$F_{p, \max} = .6528 W_p = 91.6 \#/'$

$F_{p, \min} = .1224 W_p = 17.2 \#/'$

(PER ASCE 7-05)
 CH 13.4

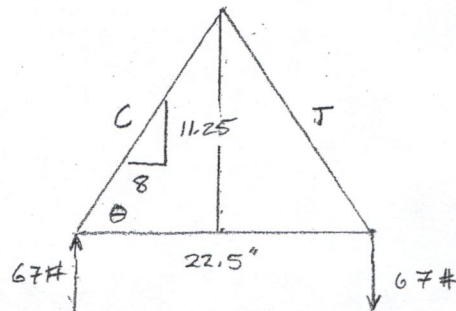
$M_{OT} = \frac{17.2 \#/' \times 7.5^2}{2} + \frac{12.4 \#/' \times 7.5^2}{3} = 717 \#'$, $1.3 (717 \#') =$

$M_{RB} = (.9 - .2 S_{ps}) W_p \frac{L}{2}$
 $= 808 \#'$

WHERE: $W_p = \frac{17}{12} (\text{DEPTH}) \times \frac{90}{12} (\text{HEIGHT}) \times 3' (\text{WIDTH}) \times 3 \frac{3}{4}$
 $= 1052 \#$

$UPLIFT = \frac{717 \#' \times 1.3 - 808 \#'}{L} = 67 \#$

$SHEAR \text{ AT BASE} = 132 \# \times 1.3 = 172 \#$



CONNECTIONS

ANCHOR BOLTS - SIMPSON TITEN HD

DEMAND VS CAPACITY

TENSION

67# < 775# OK

SHEAR

172#/2 < 1740# OK

INTERACTION EQU.

$$\left(\frac{167}{775}\right)^{5/3} + \left(\frac{86}{1740}\right)^{5/3} = .03 \leq 1.0 \quad \text{OK}$$

SHEET METAL SCREWS

DEMAND VS CAPACITY

328# LOWER OF { 390# TABLE 4 ∴
497# TABLE 5

328# < 390# OK

**TABLE 3—ALLOWABLE TENSION LOADS FOR TITEN HD SCREW ANCHORS
INSTALLED IN NORMAL-WEIGHT CONCRETE^{1,2,3,4} (pounds)**

ANCHOR DIAMETER (inch)	DRILL BIT DIAMETER ⁵ (inches)	MINIMUM EMBEDMENT DEPTH (inches)	CRITICAL EDGE DISTANCE (inches)	CRITICAL SPACING (inches)	INSTALLED WITH SPECIAL INSPECTION ⁶			INSTALLED WITHOUT SPECIAL INSPECTION ⁷		
					Concrete Strength, f'_c (psi)			Concrete Strength, f'_c (psi)		
					2,000	3,000	4,000	2,000	3,000	4,000
$\frac{3}{8}$	$\frac{3}{8}$	$2\frac{3}{4}$	3	6	1,075	1,315	1,550	540	660	775
		$3\frac{3}{4}$			1,770	2,115	2,455	885	1,060	1,230
$\frac{1}{2}$	$\frac{1}{2}$	$2\frac{3}{4}$	4	8	1,155	1,400	1,645	580	700	825
		$3\frac{5}{8}$			1,855	2,270	2,685	930	1,135	1,345
		$5\frac{3}{4}$			2,570	3,240	3,910	1,285	1,620	1,955
$\frac{5}{8}$	$\frac{5}{8}$	$2\frac{3}{4}$	5	10	1,155	1,400	1,645	580	700	825
		$4\frac{1}{8}$			2,185	2,630	3,070	1,095	1,315	1,535
		$5\frac{3}{4}$			3,240	3,955	4,670	1,620	1,980	2,335
$\frac{3}{4}$	$\frac{3}{4}$	$2\frac{3}{4}$	6	12	1,170	1,410	1,645	585	705	825
		$4\frac{5}{8}$			2,585	3,470	4,355	1,295	1,735	2,180
		$5\frac{3}{4}$			3,440	4,055	4,670	1,720	2,030	2,335

For SI: 1 inch = 25.4 mm, 1 lb = 4.45 N, 1 psi = 6.89 kPa.

¹For wind or earthquake loading conditions, allowable loads may be adjusted in accordance with Section 5.0.

²Minimum concrete thickness is 1.5 times the anchor embedment.

³Minimum edge distance and spacing shall comply with Table 1.

⁴The tabulated loads are for anchors installed in normal-weight concrete having a minimum compressive strength as shown in the table at the time of anchor installation.

⁵Anchor installation shall comply with Section 4.0 of this report.

⁶These tension loads are applicable only when the anchors are installed with special inspection as set forth in Section 4.2 of this report.

⁷These tension loads are applicable when the anchors are installed without special inspection.

**TABLE 4—ALLOWABLE SHEAR LOADS FOR TITEN HD SCREW ANCHORS
INSTALLED IN NORMAL-WEIGHT CONCRETE^{1,2,3,4} (pounds)**

ANCHOR DIAMETER (inch)	DRILL BIT DIAMETER ⁵ (inch)	MINIMUM EMBEDMENT DEPTH (inches)	CRITICAL EDGE DISTANCE (inches)	CRITICAL SPACING (inches)	ALLOWABLE SHEAR LOAD ⁶ (lbs)		
					Concrete Strength, f'_c (psi)		
					2,000	3,000	4,000
$\frac{3}{8}$	$\frac{3}{8}$	$2\frac{3}{4}$	$4\frac{1}{2}$	6	1,585	1,665	1,740
		$3\frac{3}{4}$			1,595	1,670	1,740
$\frac{1}{2}$	$\frac{1}{2}$	$2\frac{3}{4}$	6	8	1,605	2,050	2,495
		$3\frac{5}{8}$			2,330	2,795	3,255
		$5\frac{3}{4}$			2,830	3,045	3,255
$\frac{5}{8}$	$\frac{5}{8}$	$2\frac{3}{4}$	$7\frac{1}{2}$	10	1,940	2,220	2,495
		$4\frac{1}{8}$			2,175	3,415	4,650
		$5\frac{3}{4}$			3,125	3,890	4,650
$\frac{3}{4}$	$\frac{3}{4}$	$2\frac{3}{4}$	9	12	1,960	2,415	2,865
		$4\frac{5}{8}$			2,805	4,490	6,170
		$5\frac{3}{4}$			4,950	5,560	6,170

For SI: 1 inch = 25.4 mm, 1 lb = 4.45 N, 1 psi = 6.89 kPa.

¹For wind or earthquake loading conditions, allowable loads may be adjusted in accordance with Section 5.0.

²Minimum concrete thickness is 1.5 times the anchor embedment.

³Minimum edge distance and spacing shall comply with Table 1.

⁴The tabulated loads are for anchors installed in normal-weight concrete having a minimum compressive strength as shown in the table at the time of anchor installation.

⁵Anchor installation shall comply with Section 4.0 of this report.

⁶Allowable shear loads are for installations with or without special inspection.

TABLE 2—ALL2 WABLE TE1 6ILE PULL-2 UT L2 AD6 (P_{NOTLQ}) pounds-force ^{11 21 P 41 5}

Steel $F_u = 45$ ksi Applied Factor of Safety $\Omega = P.0$								
Screw Designation	Nominal Diameter (in.)	Design thickness of member not in contact with the screw head (in.)						
		0.036	0.048	0.060	0.075	0.090	0.105	0.135
#8-18	0.164	75	100	125	157	188	220	282
#10-16	0.190	87	116	145	182	218	254	327
#12-14, #12-24	0.216	99	132	165	207	248	289	373
1/4-14	0.250	115	153	191	239	287	333	430

For **SI** inch = 25.4 mm, 1 lbf = 4.4 N, 1 ksi = 6.89 MPa.¹For tension connections, the lower of the allowable pull-out, pullover, and tension fastener strength of screw found in Tables 2, 3, and 4, respectively must be used for design.²ANSI/ASME standard screw diameters were used in the calculations and are listed in the tables.³The allowable pull-out capacity for other member thicknesses can be determined by interpolating within the table.⁴To calculate LRFD values, multiply values in table by the ASD safety factor of 3.0 and multiply again with the LRFD Φ factor of 0.5.⁵For $F_u = 65$ ksi steel, multiply values by 1.44.TABLE 3—ALL2 WABLE TE1 6ILE PULL-2 9E5 L2 AD6 (P_{NOVLQ}) pounds-force ^{11 21 P 4, 5}

Steel $F_u = 45$ ksi Applied Factor of Safety $\Omega = P.0$									
Screw Designation	Washer Head Diameter (in.)	Design thickness of member in contact with the screw head (in.)							
		0.030	0.036	0.048	0.060	0.075	0.090	0.105	0.135
#8-18	0.335	225	271	363	453	567	680	790	1020
#10-16	0.399	268	323	430	540	673	807	943	1210
#12-14, #12-24	0.415	279	337	447	560	700	840	980	1260
1/4-14	0.500	336	407	540	677	843	1010	1180	1520

For **SI** inch = 25.4 mm, 1 lbf = 4.4 N, 1 ksi = 6.89 MPa.¹For tension connections, the lower of the allowable pull-out, pullover, and tension fastener strength of screw found in Tables 2, 3, and 4, respectively must be used for design.²ANSI/ASME standard screw head diameters were used in the calculations and are listed in the tables.³The allowable pull-over capacity for other member thicknesses can be determined by interpolating within the table.⁴To calculate LRFD values, multiply values in table by the ASD safety factor of 3.0 and multiply again with the LRFD Φ factor of 0.5.⁵For $F_u = 65$ ksi steel, multiply values by 1.44.

TABLE 4—) A6TE1E5 6T5E1GTH 2) 6C5EW

Screw Designation	Diameter (in.)	Allowable Fastener Strength ⁴		Nominal Fastener Strength (tested)	
		Tension (P_{ts}/Ω) ¹ (lb)	Shear (P_{ss}/Ω) ^{2,3} (lb)	Tension, P_{ts} (lb)	Shear, P_{ss} (lb)
#8-18	0.164	335	390	1000	1170
#10-16	0.190	455	405	1370	1215
#12-14	0.216	775	625	2325	1880
#12-24	0.216	1300	760	3900	2285
1/4-14	0.250	1525	815	4580	2440

For **SI**: 1 inch = 25.4 mm, 1 lbf = 4.4 N, 1 ksi = 6.89 MPa.¹For tension connections, the lower of the allowable pull-out, pullover, and tension fastener strength of screw found in Tables 2, 3, and 4, respectively must be used for design.²For shear connections, the lower of the allowable shear fastener strength and allowable shear (bearing) found in Tables 4 and 5, respectively must be used for design.³See Section 4.1 for fastener spacing and end distance requirements.⁴To calculate LRFD values, multiply the allowable fastener strengths by the ASD safety factor of 3.0 and multiply again by the LRFD Φ factor of 0.5.

SHEAR

TABLE 5—ALL 2 WABLE 6 HEA5 (BEA511 G) CAPACITY 2) C2 LD-) 2 50 ED 6 TEEL, Φ ^{11 21 P, 4, 5}

Steel $F_u = 45$ ksi Applied Factor of Safety $\Omega = P.0$									
Screw Designation	Diameter (in.)	Design thickness of member in contact with screw head, (in.)	Design thickness of member not in contact with the screw head (in.)						
			0.036	0.048	0.060	0.075	0.090	0.105	0.135
#8	0.164	0.036	174	239	239	239	239	239	239
		0.048	174	268	319	319	319	319	319
		0.060	174	268	373	400	400	400	400
		0.075	174	268	373	497	497	497	497
		0.090	174	268	373	497	597	597	597
		0.105	174	268	373	497	597	697	697
		0.135	174	268	373	497	597	697	897
#10	0.190	0.036	188	277	277	277	277	277	277
		0.048	188	289	370	370	370	370	370
		0.060	188	289	403	463	463	463	463
		0.075	188	289	403	563	577	577	577
		0.090	188	289	403	563	693	693	693
		0.105	188	289	403	563	693	807	807
		0.135	188	289	403	563	693	807	1040
#12	0.216	0.036	200	309	315	315	315	315	315
		0.048	200	308	420	420	420	420	420
		0.060	200	308	430	523	523	523	523
		0.075	200	308	430	600	657	657	657
		0.090	200	308	430	600	787	787	787
		0.105	200	308	430	600	787	920	920
		0.135	200	308	430	600	787	920	1180
1/4 in.	0.250	0.036	215	340	363	363	363	363	363
		0.048	215	331	467	487	487	487	487
		0.060	215	331	463	607	607	607	607
		0.075	215	331	463	647	760	760	760
		0.090	215	331	463	647	850	910	910
		0.105	215	331	463	647	850	1060	1060
		0.135	215	331	463	647	850	1060	1370

For SIW inch = 25.4 mm, 1 lbf = 4.4 N, 1 ksi = 6.89 MPa.

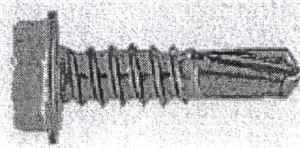
¹The lower of the allowable shear fastener strength and shear bearing found in Tables 4 and 5, respectively must be used for design.

²ANSI/ASME standard screw diameters were used in the calculations and are listed in the tables

³The allowable bearing capacity for other member thicknesses can be determined by interpolating within the table.

⁴To calculate LRFD values, multiply values in table by the ASD safety factor of 3.0 and multiply again with the LRFD Φ factor of 0.5.

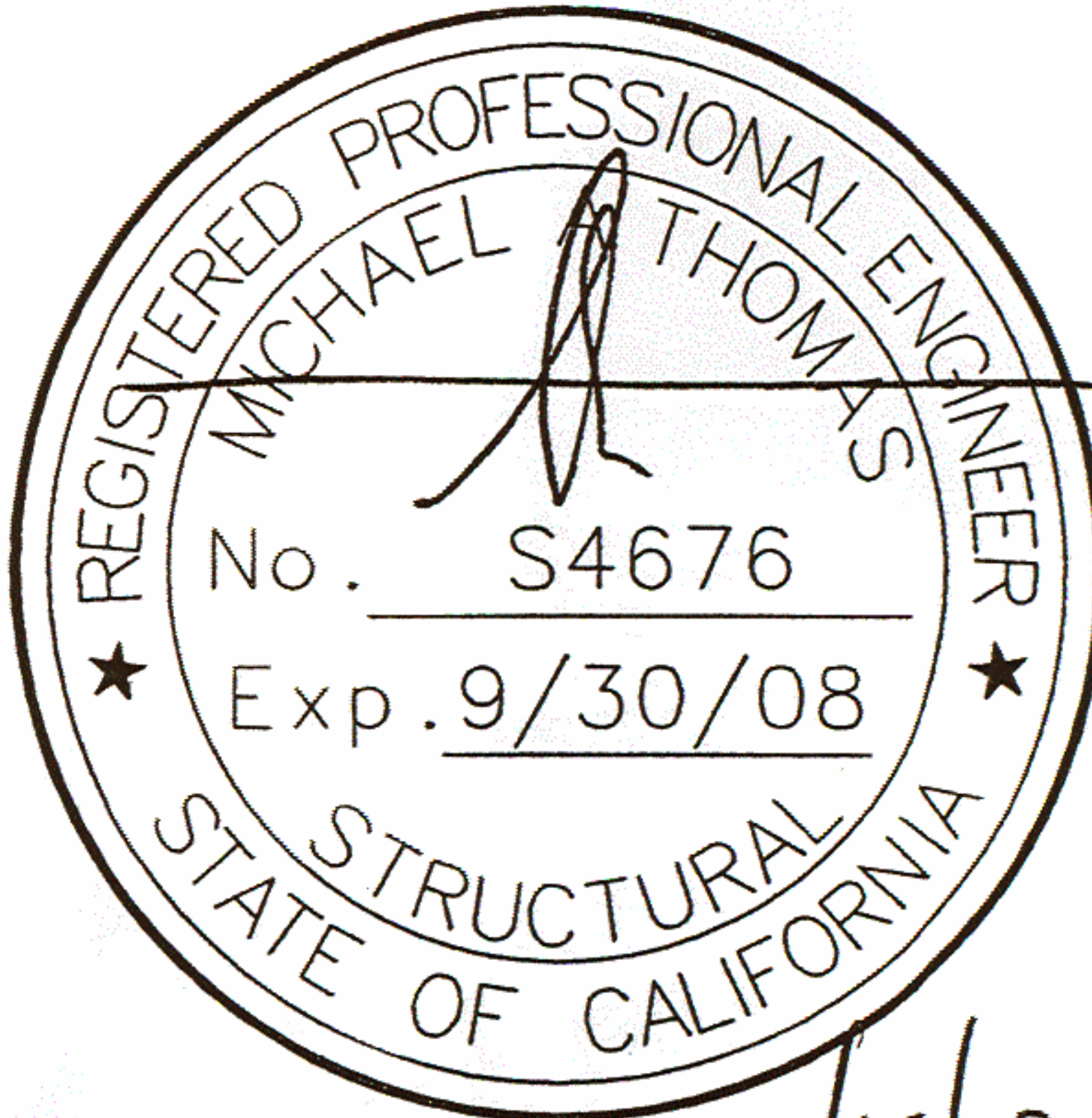
⁵For $F_u = 65$ ksi steel, multiply values by 1.44.



) IGU5 E 1—HILTI HEX WA6 HE5 HEAD 6 EL) -D5 ILLI1 G 6 C5 EW

PLANS FOR CONSTRUCTION
SELMA BRANCH LIBRARY
ANCHORING OF BOOK SHELVING
2200 SELMA STREET
SELMA, CALIFORNIA 93662

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DEPARTMENT OF
PUBLIC WORKS
AND PLANNING

CAPITAL PROJECTS DIVISION
2220 Tulare St. Suite 720, Fresno, California 93721
Phone: (559) 262-4078 Fax: (559) 262-4879

PROJECT:
**BRANCH LIBRARIES
ANCHORING OF BOOK SHELVING
SELMA BRANCH**

SHEET CONTENT:
**GENERAL NOTES
AND
SPECIFICATIONS**

DRAWN BY: MLT CHECKED BY: RBS
ISSUE DATE: 1.21.08 CONTRACT NO: 2002025F

ARCHIVE NAME:
2002025FS1

SHEET NUMBER:
S-1
SHT. 1 OF 3 SHEETS

SPECIFICATIONS:

GENERAL

ALL WORK SHALL CONFORM TO THE REQUIREMENTS OF THE 2007 EDITION OF THE CALIFORNIA BUILDING CODE

STEEL SHEET METAL

- A. PROVIDE MATERIALS SELECTED FOR THEIR SURFACE FLATNESS, SMOOTHNESS, AND FREEDOM FROM SURFACE BLEMISHES. DO NOT USE MATERIALS WHOSE EXPOSED SURFACES EXHIBIT PITTING, SEAM MARKS, ROLLER MARKS, ROLLED TRADE NAMES, ROUGHNESS, AND VARIATIONS IN FLATNESS. SHEAR AND PUNCH METALS CLEANLY AND ACCURATELY. REMOVE BURRS.
- B. STEEL SHEET SHALL CONFORM TO ASTM A653.
- C. ROLLED STELL FLOOR PLATES: ASTM A786.
- D. BOLTS, NUTS, WASHERS AND SHEET METAL SCREWS (SMS) SHALL CONFORM TO ASTM A153.
- E. PAINTING SHALL BE DONE BY THE SHEET STEEL FABRICATOR. THE SHEET STEEL SHALL BE PAINTED WITH STANDARD, FAST-CURING, LEAD FREE, UNIVERSAL MODIFIED ALKYD PRIMER FOLLOWED BY A LATEX SEMI-GLOSS TOPCOAT OF THE 'SPCC: THE SOCIETY FOR PROTECTIVE COATINGS. FINAL TOPCOAT COLOR SHALL MATCH THE EXISTING BOOK SHELVING COLOR.

SCREW ANCHORS

SCREW ANCHORS SHALL BE SIMPSON TITEN HD ANCHORS OR APPROVED EQUAL. ANCHORS SHALL BE INSTALLED IN ACCORDANCE WITH THE ICC REPORT ESR-1056. ANCHORS SHALL USE WASHERS SIZED TO PREVENT CRUSHING OF THE ATTACHED MEMBER AT INSTALLATION TORQUE.

CONSTRUCTION LIABILITY

THE CONSTRUCTION CONTRACTOR AND HIS SUBCONTRACTORS AGREE THAT IN ACCORDANCE WITH GENERALLY ACCEPTED CONSTRUCTION PRACTICES, THE CONSTRUCTION CONTRACTOR AND HIS SUBCONTRACTORS WILL BE REQUIRED TO ASSUME SOLE AND COMPLETE RESPONSIBILITY FOR JOB SITE CONDITIONS DURING THE COURSE OF CONSTRUCTION OF THE PROJECT, INCLUDING SAFETY OF ALL PERSONS AND PROPERTY; THAT THIS REQUIREMENT SHALL BE MADE TO APPLY CONTINUOUSLY AND NOT LIMITED TO NORMAL WORKING HOURS, AND THE CONSTRUCTION CONTRACTOR AND HIS SUBCONTRACTORS FURTHER AGREE TO DEFEND, INDEMNIFY & HOLD THE DESIGN PROFESSIONAL HARMLESS FROM ANY AND ALL LIABILITY, REAL OR ALLEGED, IN CONNECTION WITH THE PERFORMANCE OF WORK ON THIS PROJECT, EXCEPT LIABILITY ARISING FROM THE SOLE NEGLIGENCE OF THE DESIGN PROFESSIONAL.

SUBMITTALS

1. METAL FINISHED SAMPLE PLATE FOR FRESNO COUNTY REVIEW.

TESTING AND SPECIAL INSPECTIONS:

THE OWNER SHALL EMPLOY A SPECIAL INSPECTOR DURING CONSTRUCTION ON THE FOLLOWING TYPES OF WORK:

SCREW ANCHORS

THE SPECIAL INSPECTOR SHALL VERIFY THE FOLLOWING AND RECORD THE INFORMATION IN THE INSPECTION REPORT:

- ANCHOR TYPE, SIZE AND DIMENSIONS
- HOLE DIMENSIONS AND CLEANLINESS
- ANCHOR SPACING
- EDGE DISTANCES
- ANCHOR EMBEDMENT

SPECIAL INSPECTOR

- THE SPECIAL INSPECTOR SHALL BE A QUALIFIED PERSON WHO SHALL DEMONSTRATE HIS COMPETENCE, TO THE SATISFACTION OF THE BUILDING OFFICIAL, FOR INSPECTION OF A PARTICULAR TYPE OF CONTRUCTION OR OPERATION REQUIRING SPECIAL INSPECTION.

DRAWING INDEX

- S-1 GENERAL NOTES AND SPECIFICATONS
S-2 BOOK SHELVING FLOOR PLAN
S-3 BOOK SHELVING SECTIONS AND DETAILS

ADOPTED BY THE
FRESNO COUNTY BOARD OF SUPERVISORS

FEBRUARY 26, 2008

Henry Perea Chairman 3rd District

Susan Anderson Vice Chairman 2nd District

Phil Larson 1st District

Judy Case 4th District

Bob Waterston 5th District

Bart B. Bohn

Alan Weaver Director,
Department of Public Works and Planning

FEB 21 2008

DEVELOPMENT SERVICES
County Administrative Officer

ARCHITECT / ENGINEER: PUBLIC WORKS AND PLANNING DEPARTMENT

NAME: CAPITAL PROJECTS DIVISION

ADDRESS: 2220 TULARE STREET, SUITE 720

CITY: FRESNO STATE: CALIF. ZIP CODE: 93721 PHONE: (559) 262-4267

OWNER: COUNTY OF FRESNO, 2281 TULARE ST., FRESNO, CA. 93721

PROJECT ADDRESS: 2200 SELMA STREET

CITY: SELMA STATE: CALIF. ZIP CODE: 93662 PHONE: (559) 896-3393

ASSESSOR'S PARCEL No.: 389-191-08T, 09T

TYPE OF REMODEL USAGE: LIBRARY

AREA OF BUILDING: 7,432 S.F.

OCCUPANCY GROUPS: N/A

OCCUPANT LOAD: N/A

FRESNO COUNTY BUILDING NUMBER: 804

CALIFORNIA CONTRACTOR'S LICENSE REQUIRED FOR THIS PROJECT:

CLASS B, GENERAL BUILDING CONTRACTOR

CONTRACT NUMBER:

2008-S-XX

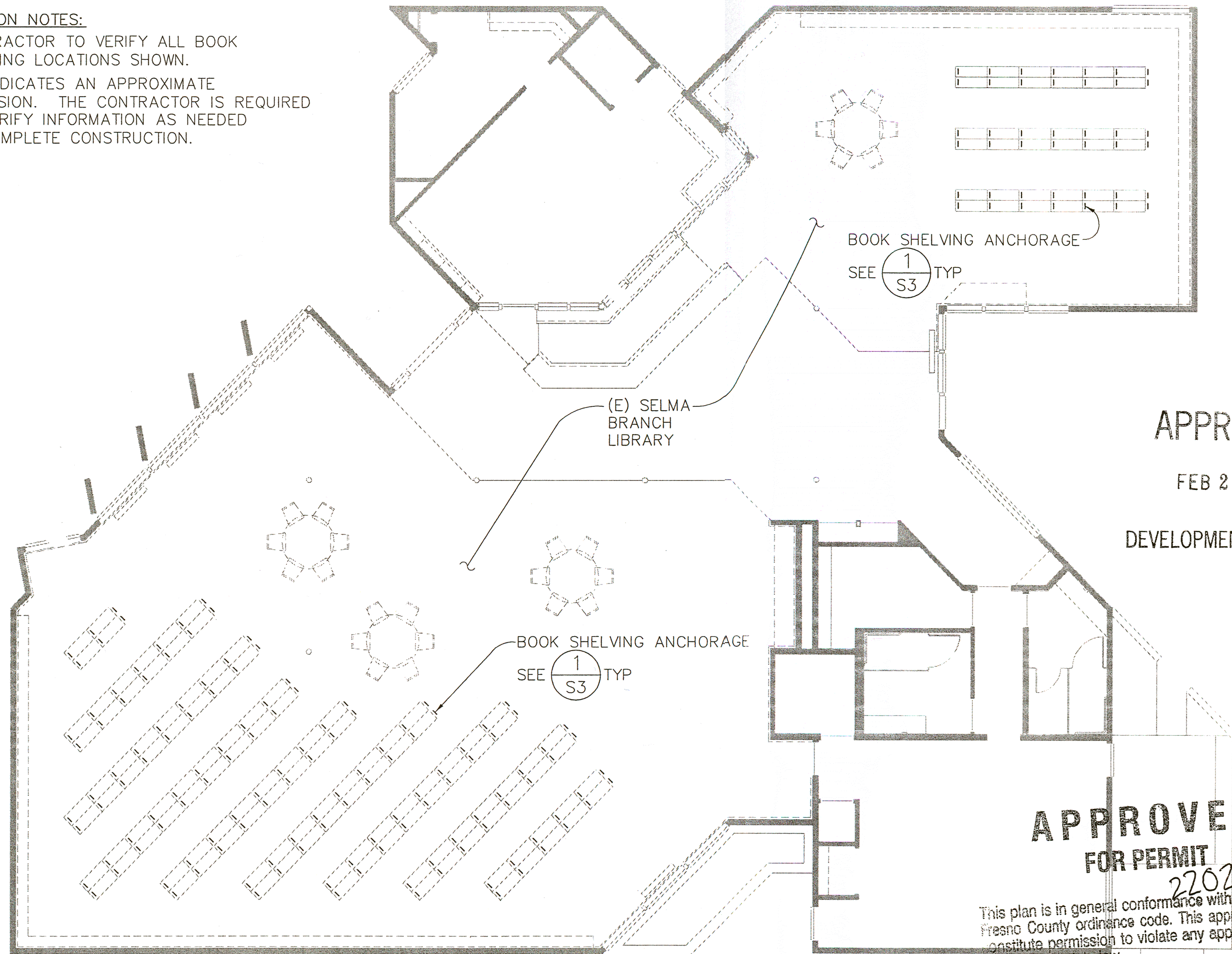
FUND / SUBCLASS / ORG / ACCOUNT NUMBER:

0107 / 10000 / 75110700 / 7385

(2002025FS1) 2002025F

FOUNDATION NOTES:

1. CONTRACTOR TO VERIFY ALL BOOK SHELVING LOCATIONS SHOWN.
2. ± INDICATES AN APPROXIMATE DIMENSION. THE CONTRACTOR IS REQUIRED TO VERIFY INFORMATION AS NEEDED TO COMPLETE CONSTRUCTION.



APPROVED

FEB 21 2008

DEVELOPMENT SERVICES

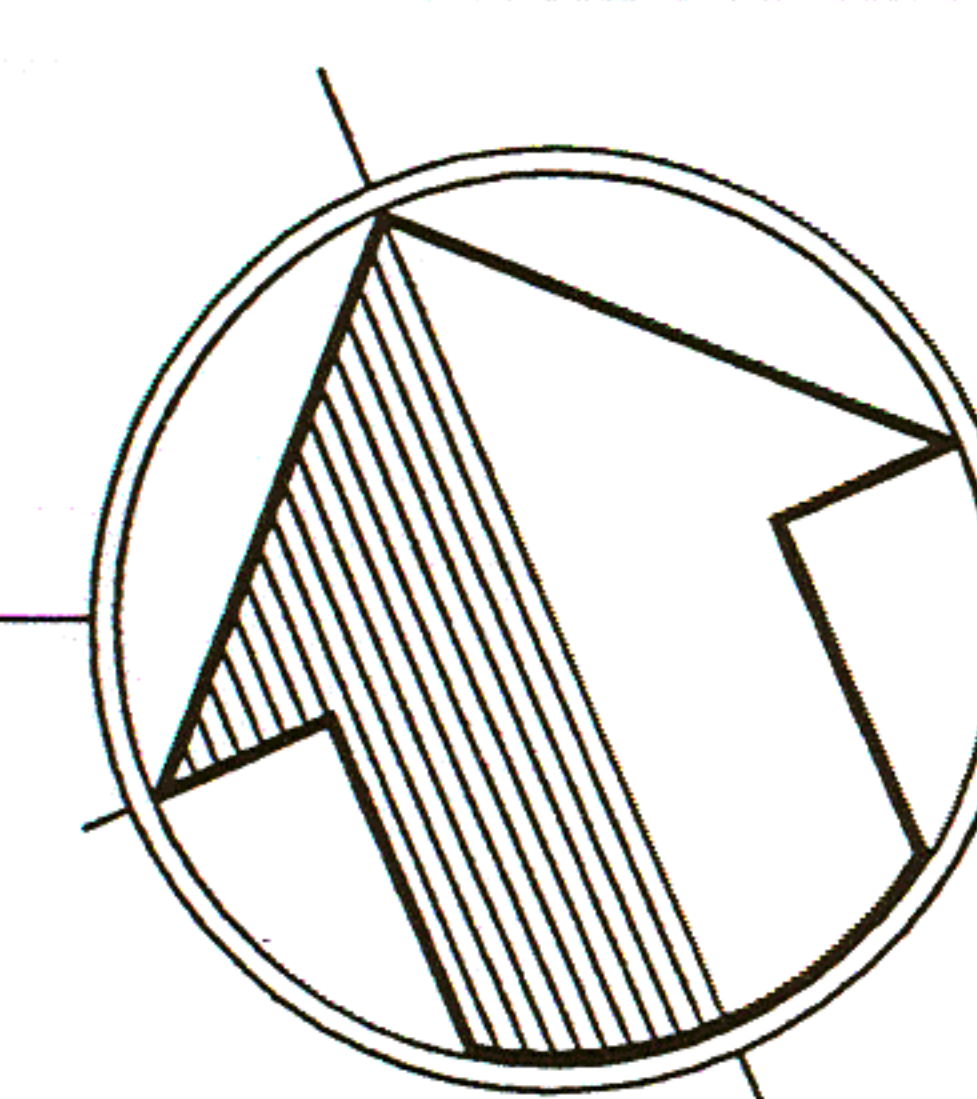
APPROVED
FOR PERMIT

This plan is in general conformance with title 15 of the Fresno County ordinance code. This approval does not constitute permission to violate any applicable county ordinance or state law.

By *[Signature]* 2/21/08
COUNTY OF FRESNO DATE

BOOK SHELVING FLOOR PLAN

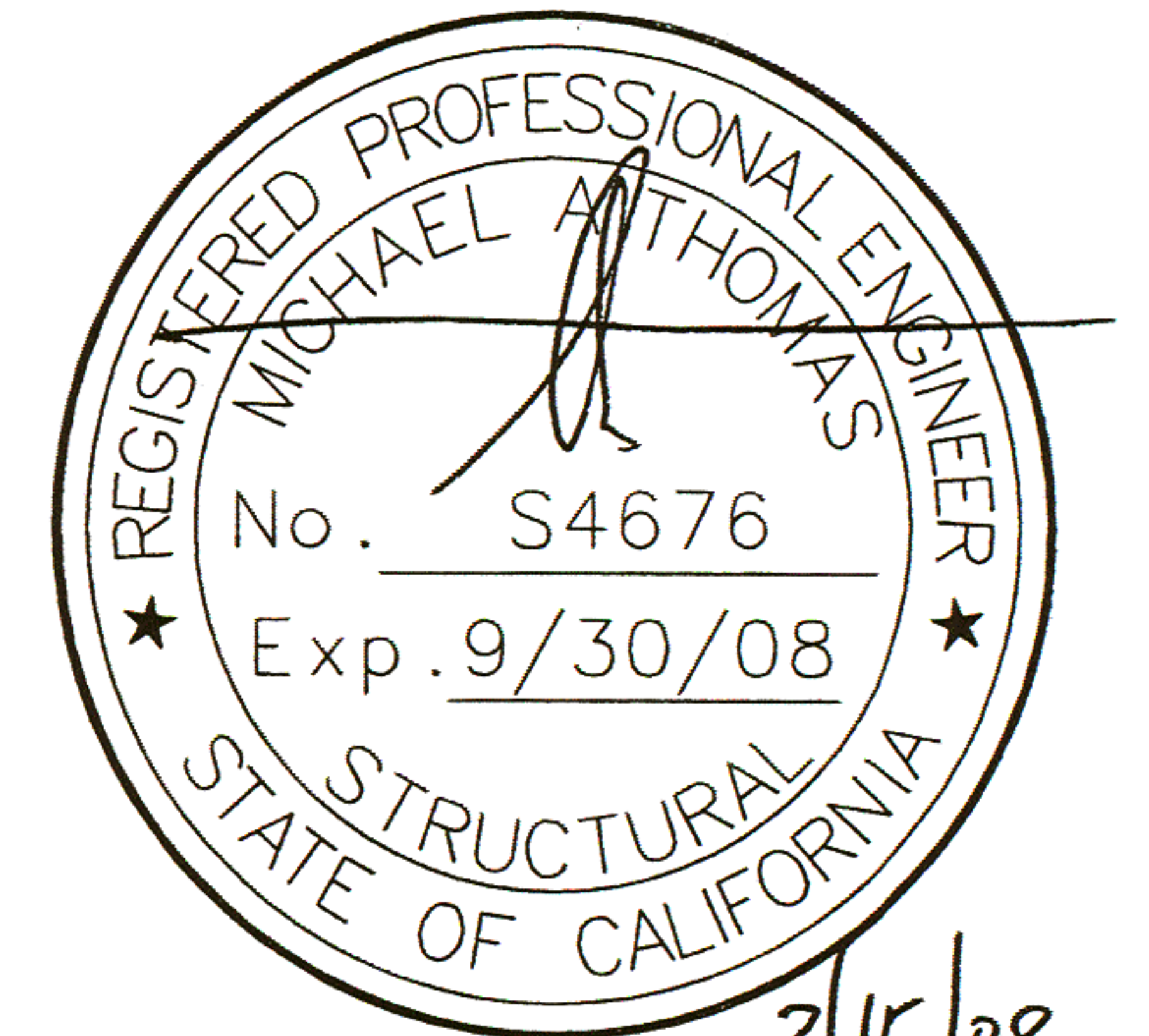
3/32" = 1'-0"



BIGGS CARDOSA ASSOCIATES INC
STRUCTURAL ENGINEERS

BCA

5250 N. Palm Avenue, Suite 211
Fresno, California 93704
559-449-8686



ATTENTION:
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DEPARTMENT OF
PUBLIC WORKS
AND PLANNING

CAPITAL PROJECTS DIVISION
2220 Tulare St. Suite 720, Fresno, California 93721
Phone: (559) 262-4078 Fax: (559) 262-4879

PROJECT:

BRANCH LIBRARIES
ANCHORING OF BOOK SHELVING
SELMA BRANCH

SHEET CONTENT:

BOOK SHELVING
FLOOR PLAN

DRAWN BY: MLT CHECKED BY: RBS
ISSUE DATE: 1.21.08 CONTRACT NO: 2002025F

ARCHIVE NAME: 2002025FS2

SHEET NUMBER:

S-2

SHT. 2 OF 3 SHEETS

(2002025FS2) 2002025F

